



## Analysis of the influence of chlorides on the mechanical behaviour of HPSFRCC

Mylene M. Vieira<sup>a, b, \*</sup>, Sergio H. P. Cavalaro<sup>c</sup>, Antonio Aguado<sup>d</sup>

<sup>a</sup> Department of Civil Engineering, Federal University of Ceará, Campus Russas, 411 Felipe Santiago Street, 62900-000, Russas, Brazil.

<sup>b</sup> CAPES Foundation, Ministry of Education of Brazil, Brasília - DF, 70040-020, Brazil.

<sup>c</sup> School of Architecture, Building and Civil Engineering, Loughborough University, Leicestershire, UK.

<sup>d</sup> Department of Civil and Environmental Engineering, Barcelona Tech, Polytechnic University of Catalonia, UPC, Jordi Girona 1-3, 08034, Barcelona, Spain.

### Article info

Received 12 February 2021.

Received in revised form 14  
April 2021

Accepted 8 May 2021

### Keywords

HPSFRCC

Steel fibre

Chlorides

Corrosion

Mechanical properties

### Abstract

High-performance steel fibre reinforced cementitious composites (HPSFRCC) present optimized mechanical properties. Due to the large amount of randomly distributed steel fibres in the cementitious matrix, the composite aesthetic aspect and mechanical response may be damaged in elements subject to chloride exposure. The objective of this research is to evaluate the mechanical behaviour of uncracked HPSFRCC specimens subjected to chloride exposure. Eight mixes of HPSFRCCs with different fibre contents (40, 80, 120 and 160 kg/m<sup>3</sup>) and with the addition of chlorides were designed. Mechanical properties of the composites were evaluated by 3-point bending tests and the presence of corrosion products in the specimen cross-section was analysed by visual inspection. Results showed that the chloride added to the mixes has little influence on the post-cracking response of the composite.

## Análise da influência de cloretos no comportamento mecânico de compósitos cimentícios de alto desempenho reforçados com fibras de aço

### Informações

Recebido 12 Fevereiro 2021

Manuscrito revisado recebido  
14 Abril 2021

Aceito 8 Maio 2021

### Palavras-chave

HPSFRCC

Fibra de aço

Cloretos

Corrosão

Propriedades mecânicas

### Resumo

Compósitos cimentícios de alto desempenho reforçados com fibra de aço (HPSFRCC) apresentam propriedades mecânicas otimizadas. Devido à grande quantidade de fibras de aço distribuídas aleatoriamente na matriz cimentícia, o aspecto estético do compósito e a resposta mecânica podem ser prejudicados em elementos sujeitos à exposição a cloretos. O objetivo deste trabalho é avaliar o comportamento mecânico de corpos de prova de HPSFRCC não fissurados submetidos à exposição a cloreto. Foram produzidas oito misturas de HPSFRCCs com diferentes teores de fibra (40, 80, 120 e 160 kg/m<sup>3</sup>) e com adição de cloretos. As propriedades mecânicas dos compósitos foram avaliadas por ensaio de flexão de 3 pontos e a presença de produtos de corrosão na seção transversal da amostra foi analisada por inspeção visual. Os resultados mostraram que o cloreto adicionado às misturas tem pouca influência na resposta pós-fissuração dos compósitos.

## Análisis de la influencia de los cloruros en el comportamiento mecánico de compósitos cementícios de alto desempeño reforzados con fibras de acero

### Información

Recibido 12 Febrero 2021

Manuscrito revisado  
recibido 14 Abril 2021

Aceptado 8 Mayo 2021

### Palabras clave

HPSFRCC

Fibra de acero

Cloruros

Corrosión

Propiedades mecânicas

### Resumen

Compósitos cementícios de alto desempeño reforzados con fibra de acero (HPSFRCC) tienen propiedades mecânicas optimizadas. Debido a la gran cantidad de fibras de acero distribuidas aleatoriamente en la matriz cementicia, el aspecto estético del compósito y la respuesta mecânica pueden verse perjudicados en elementos sujetos a exposición a cloruros. El objetivo de este trabajo es evaluar el comportamiento mecânico de muestras de HPSFRCC no fisuradas sometidas a exposición a cloruro. Se produjeron ocho mezclas de HPSFRCC con diferentes contenidos de fibra (40, 80, 120 y 160 kg/m<sup>3</sup>) y con la adición de cloruros. Las propiedades mecânicas de los materiales compósitos se evaluaron mediante el ensayo de flexión de 3 puntos y la presencia de productos de corrosión en la sección transversal de la probeta se analizó mediante inspección visual. Los resultados mostraron que el cloruro agregado a las mezclas tiene poca influencia en la respuesta post-fisuración de los compósitos.

\* Corresponding author: Department of Civil Engineering, Federal University of Ceará, Campus Russas, 62900-000, Russas, Brazil.

E-mail address: mylene.melo@ufc.br (Mylene M. Vieira, ORCID 0000-0002-9321-0706)

<https://doi.org/10.47842/juts.v4i1.29>

ISSN: 2675-780X

## 1. Introduction

Steel fibre reinforced concrete (SFRC) is a composite with optimized post-crack behaviour provided by the addition of fibres in the brittle cementitious matrix. For structural applications, high-modulus fibres can be used to substitute, partially or totally, conventional reinforcement (DI PRISCO; PLIZZARI; VANDEWALLE, 2009). Several researchers have highlighted the improvement of mechanical behaviour in SFRC structural properties and applications (ALVAREZ, 2013; DI PRISCO, 2009; NAAMAN, 2004; RIZZUTI; BENCARDINO, 2014; THOMAS; RAMASWAMY, 2007).

Steel fibre in uncracked specimens presents a superior resistance to corrosion compared to conventional rebar due to the smaller dimensions of the fibre that favours the fibre-matrix interface (BERROCAL; LUNDGREN; LÖFGREN, 2016; DAUBERSCHMIDT, 2006; FRAZÃO et al., 2016; NORDSTRÖM, 2005; SADEGHI-POUYA et al., 2013). Moreover, chloride threshold concentrations need to be higher to oxidize steel fibres than to oxidize steel rebars (DAUBERSCHMIDT, 2006). Studies in SFRC subjected to chloride exposure have reported corrosion limited to steel fibres up to 5 mm from the surface. Regarding accelerated chloride corrosion in high-performance steel fibre reinforced concrete (HPSFRC), only minor surface corrosion is observed (BALOUCH; FORTH; GRANJU, 2010; GRANJU; BALOUCH, 2005; SERNA; ARANGO, 2008).

Due to the minor corrosion, the effect of chloride exposure in uncracked SFRC samples leads to a negligible influence on the mechanical performance of the composite (ALIZADE; JANDAGHI ALAEE; ZABIHI, 2016). No changes on the post-crack behaviour of corroded uncracked SFRC compared to no-corroded specimens were observed after wet-dry cycles in the study of Graff et al (2009). However, the research conducted by Mantegazza and Gatti (2004) highlighted a reduction in the mechanical performance, while Kim, Boyd and Lee (2011) indicated a slight increment in the mechanical behaviour of SFRC under chloride cycles.

Specimens of HPSFRC subjected to various corrosive environments (salt ponding, immersion in 3% and 10% of chloride solution) presented

no reduction in the mechanical properties after long-term test (ABBAS, 2014). Such result was attributed to the dense microstructure and strong bond among aggregates, cementitious materials and steel fibres in the matrix. By contrast, SFRC specimens subjected to a corrosive environment (immersed in a 3.5% NaCl solution) showed a slight loss of flexural strength and toughness according to Carrillo, Pulido and Aperador (2017). These works demonstrate that the effects of chlorides on the mechanical post-crack behaviour of fibres in HPSFRCs are still unclear.

In this context, this work aims to assess the effects of corrosion on the mechanical response of uncracked HPSFRCCs specimens subjected to an accelerated chloride exposure. An experimental programme was conducted with eight mixes of HPSFRCCs produced with four different fibre contents (40, 80, 120 and 160 kg/m<sup>3</sup>) and with the addition of chloride to the mixes. The 3-point test and the visual inspection of the cross-section were performed at different ages. Results showed a small reduction on the post-crack behaviour of the composite the presence of oxidized fibres on the cross-section of the specimen, close to its surface.

## 2. Experimental methodology

### 2.1. Materials and mixes

The experimental methodology presenting the materials, mixes, specimen preparation and curing conditions for the experimental programme is described in detailed in a previous paper from the research group (VIEIRA; CAVALARO; AGUADO, 2021). Table 1 shows the composition of the 8 mixes designed for the experimental programme.

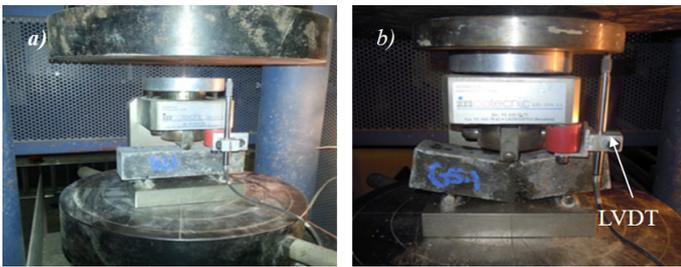
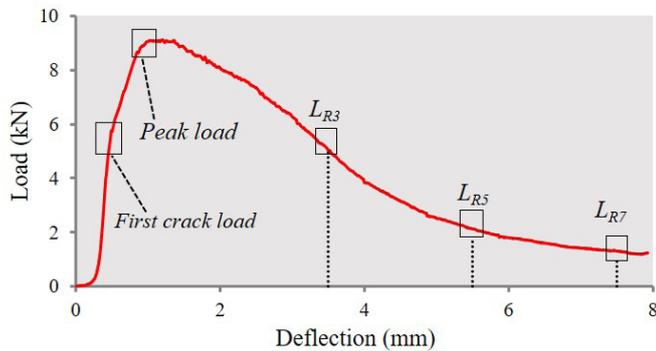
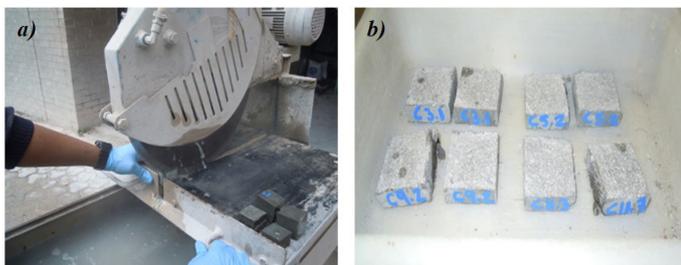
In total, 36 specimens measuring 40 mm x 40 mm x 160 mm (height x width x length) were produced per mix. Mixes with no chloride are identified by 'F\_fibre content' and mixes containing chloride are identified by 'F\_fibre content - Cl'.

### 2.2. 3-point bending test

3-point bending tests were performed according

**Table 1.** Properties of fibres used (provided by the manufacturer).

Components	F_40	F_80	F_120	F_160	F_40 - Cl	F_80 - Cl	F_120 - Cl	F_160 - Cl
CEM I 42.5 R (kg/m <sup>3</sup> )	700	700	700	700	700	700	700	700
Limestone filler (kg/m <sup>3</sup> )	255	255	255	255	255	255	255	255
Sand (0/2 mm) (kg/m <sup>3</sup> )	631.6	623	608.6	594.3	631.6	623	608.6	594.3
Sand (0/5 mm) (kg/m <sup>3</sup> )	340.1	335.4	327.7	320	340.1	335.4	327.7	320
Water (kg/m <sup>3</sup> )	213.5	213.6	210.9	208.2	213.5	213.6	210.9	208.2
Superplasticizer (kg/m <sup>3</sup> )	31.5	31.5	35	38.5	31.5	31.5	35	38.5
Retarder (kg/m <sup>3</sup> )	2.1	2.1	2.1	2.1	2.1	2.1	2.1	2.1
Steel fibre (kg/m <sup>3</sup> )	40	80	120	160	40	80	120	160
Sodium chloride (kg/m <sup>3</sup> )	-	-	-	-	44	44	44	44
Flow diameter (mm)	209	213	215	210	209	210	211	209

**Fig. 1.** 3-point bending test: (a) specimen in the apparatus for the test and (b) specimen failure after the test.**Fig. 2.** Parameters derived from the load-deflection curves.**Fig. 3.** Specimen preparation: (a) cutting of the 1-cm thick slices and (b) 2 pieces of each specimen for the inspection.

to UNE-EN 196-1 (AENOR, 2005). An Ibertest MEH 3000 kN press and a bending apparatus were used. The vertical displacement of the press was monitored by a LVDT to control the load application rate (Figure 1). For each age and mix, 4 specimens were tested at 16, 30, 49 and 109 days after casting.

Figure 2 presents the main parameters from the bending test, which are the first crack load, the peak load and the residual loads  $L_{R3}$ ,  $L_{R5}$  and  $L_{R7}$  (at a distance of 3, 5 and 7 mm from the first crack deflection, respectively). Corrosion was analysed considering the presence of chlorides, age, curing condition and fibre content.

### 2.3. Visual inspection of the cross-section

After the bending tests, 2 slices (10 mm thick) were cut from each specimen perpendicular to the cross-section of the specimen (Figure 3). After cleaning, the cross-sections of the specimens were analysed with a Stereo Microscope to identify fibre corrosion spots.

## 3. Results and discussion

### 3.1. 3-point bending test

#### 3.1.1. Load-deflection curves

Figure 4 presents the average load-deflection curves of specimens with and without chlorides for all mixes and ages. A general overview reveals that the shape of the curves of mixes with chloride is similar to that found in specimens with nochloride. Although chlorides may affect cement hydration and cause corrosion, they did not affect the shape of the curves.

Until the first crack appears, the elastic response of the composite is observed, which is governed by

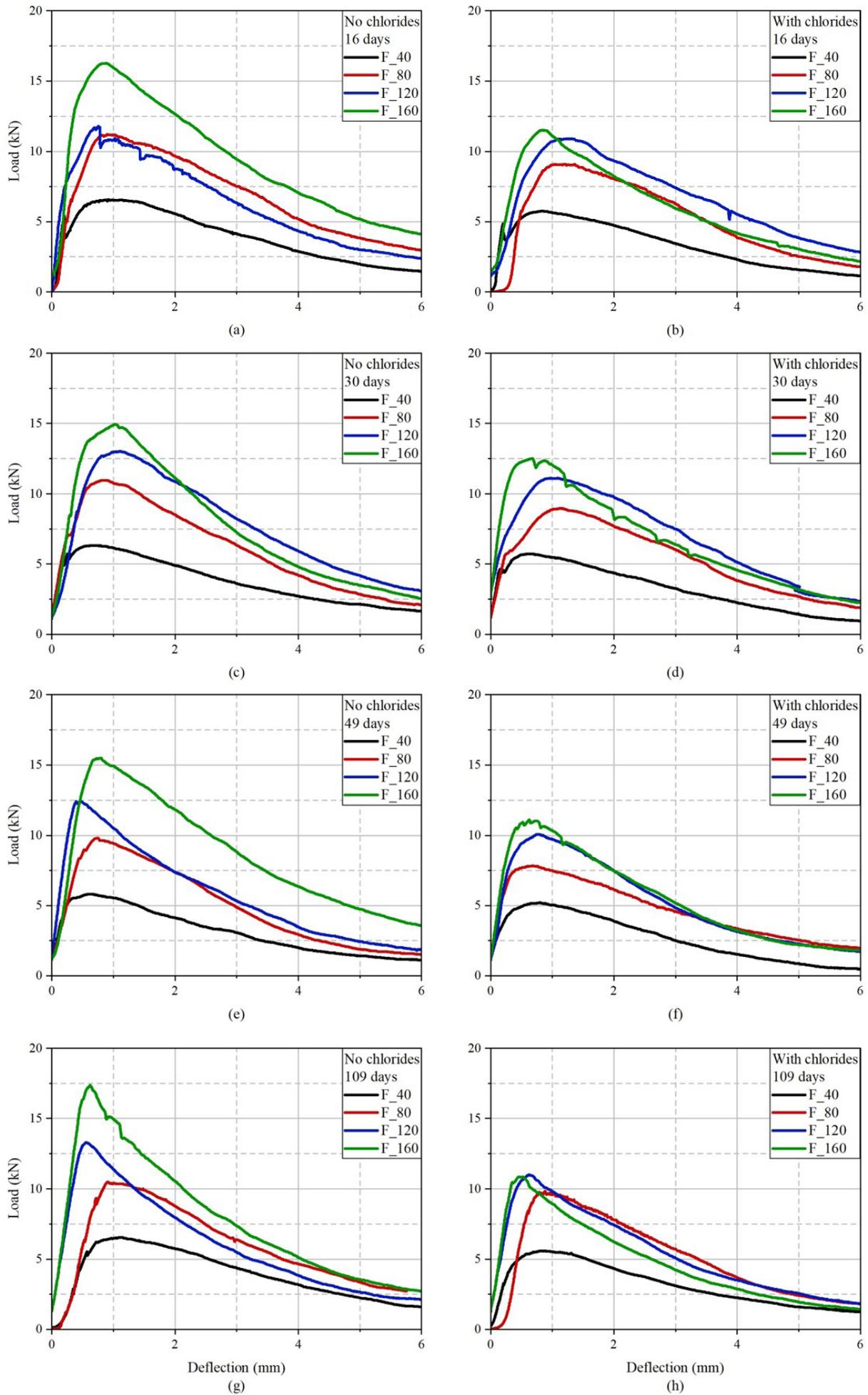


Fig. 4. Average load-deflection curves of specimens from mixes F\_40, F\_80, F\_120 and F\_160, tested at different ages.

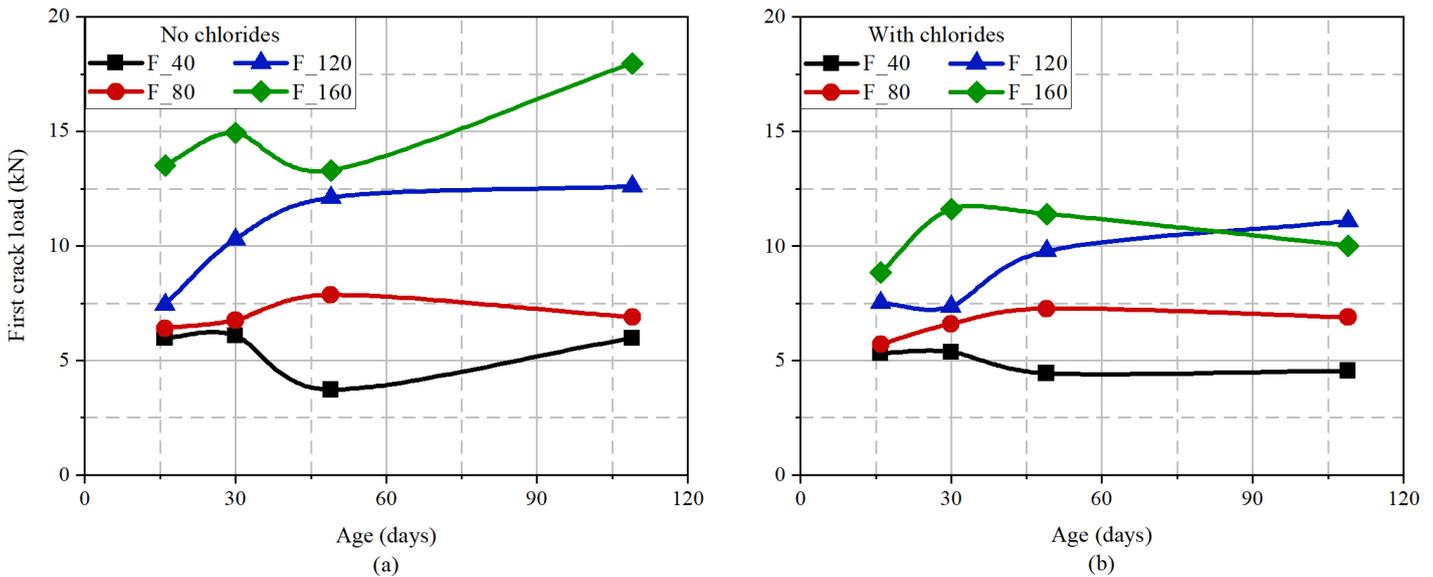


Fig. 5. First crack load of specimens from mixes F\_40 F\_80, F\_120 and F\_160: (a) specimens without chlorides and (b) specimens with chlorides.

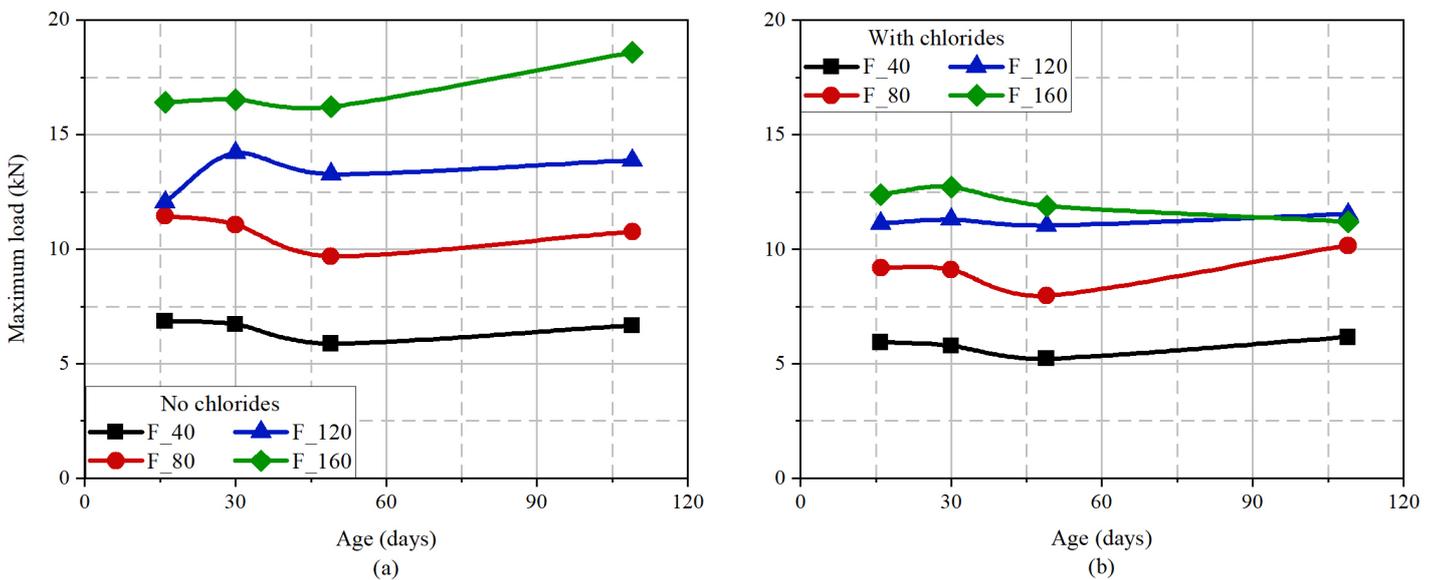


Fig. 6. Maximum load of specimens from mixes F\_40 F\_80, F\_120 and F\_160: (a) specimens without chlorides and (b) specimens with chlorides.

the matrix properties with a limited contribution of the fibres. After that, an increase of both load and displacement is observed as the load is transferred to fibres, which bridge multiple cracks, leading to a deflection-hardening behaviour. Once the peak load is reached, the localization of cracks takes place, leading to a reduction of the load-bearing capacity of the specimen.

### 3.1.2. First crack

Figure 5 shows the first crack load measured at 16, 30, 49 and 109 days for mixes F\_40, F\_80, F\_120 and F\_160 with and with no chlorides. The use of chlorides reduces the load bearing capacity of the matrix. Specimens with no chlorides present slightly

higher values of first crack load in comparison with equivalent specimens with chlorides. This may be a consequence of the change in cement hydration induced by the chlorides (TAYLOR, 1997). No clear trend on how the difference varies with the age, curing condition or fibre content is observed.

### 3.1.3. Maximum load

Figure 6 shows the average maximum load values of specimens from mixes F\_40 F\_80, F\_120 and F\_160 with and without chlorides at the ages of at 16, 30, 49 and 109 days. The maximum load is reached after multiple crack formation. At this stage, the main mechanism contributing to the load-bearing capacity of the specimen is the fibre-matrix

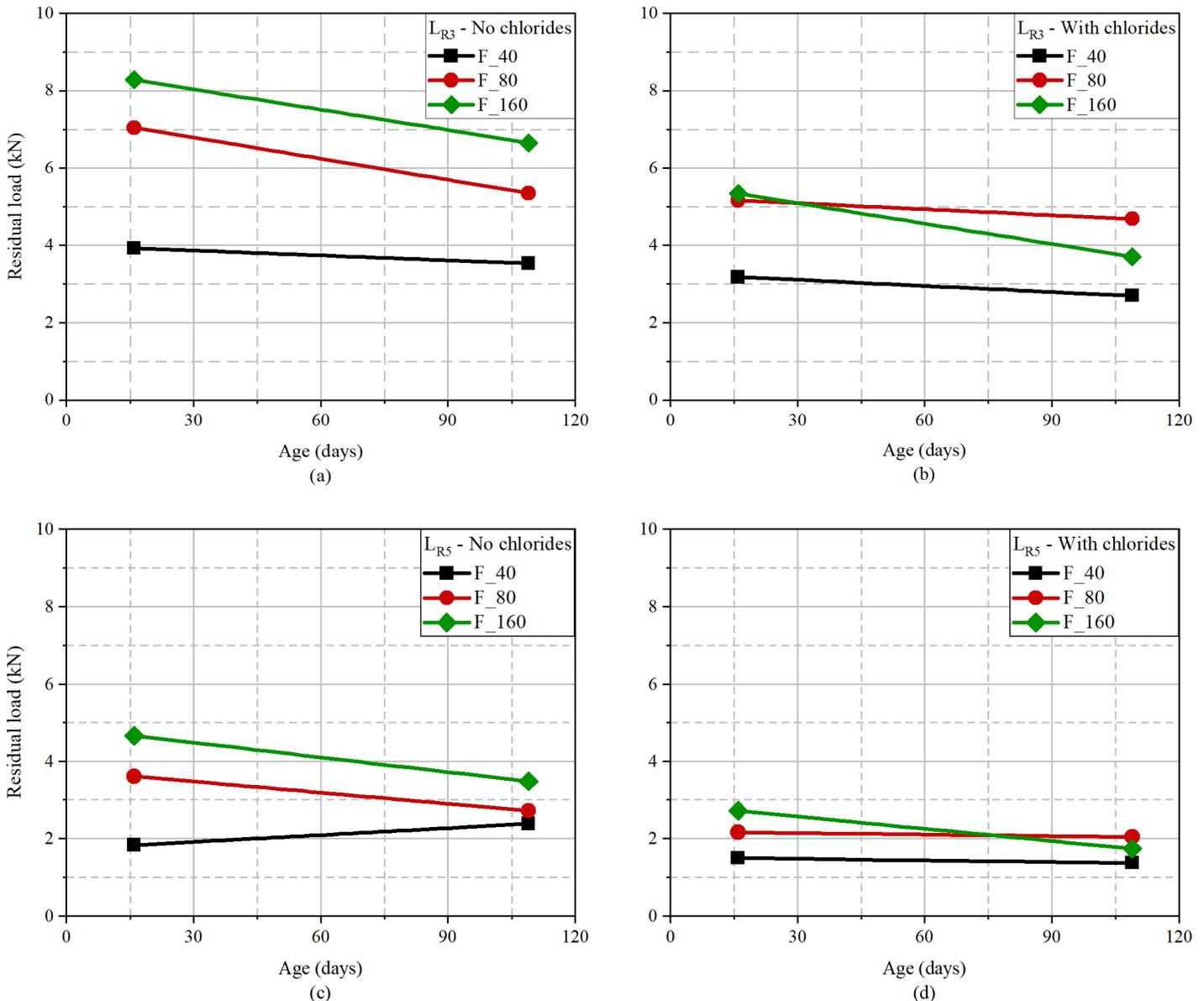


Fig. 7. Residual load of specimens from mixes F\_40 F\_80 and F\_160: (a) and (c) specimens without chlorides and (b) and (d) specimens with chlorides.

bond. Chloride addition leads to a reduction of the load-bearing capacity of the composite due to its influence on cement hydration and fibre oxidation.

Considering fibre corrosion as the main cause for the reduction on the load-bearing capacity of specimens, more significant reductions would be observed over time, increasing the difference between the maximum load obtained in mixes with and without chloride. This contradicts results from Figure 6, suggesting that the main cause of the difference in the maximum load is the influence of chlorides on cement hydration. Since no reduction in the maximum load was observed over time, fibre corrosion was not detrimental. The dense matrix due (low w/c and high cement content) limits ion diffusion needed to activate corrosion.

### 3.1.4. Residual load

Figure 7 shows the residual loads  $L_{R3}$  and  $L_{R5}$  of specimens from mixes F\_40 F\_80 and F\_160 with and without chlorides at the ages of at 16 and 109 days.  $L_{R3}$  and  $L_{R5}$  are observed after the peak-load, indicating a progressive crack opening characterized by fibre pullout.

Residual load values decrease over time, whereas the difference between specimens with and without chlorides increases with time. Such trend is more evident in mixes with higher fibre content. This suggests a possible influence of the corrosion process in the results, although definitive conclusions may not be drawn due to the influence of the chlorides in the hydration process.

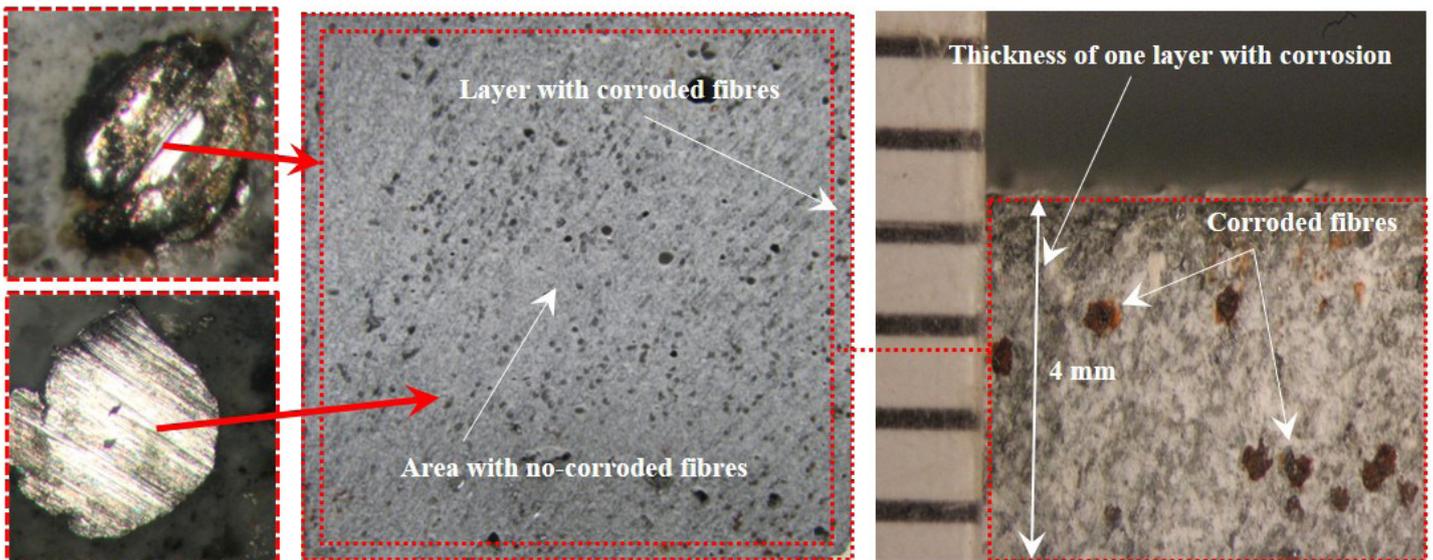


Fig. 8. Cross section of one specimen with chlorides.

### 3.2. Analysis of fibres at the specimen cross-section

Specimens were visually inspected immediately after the bending tests. Figure 8 shows the cross-section of a specimen with chlorides, indicating that only fibres close to specimen surface presented signs of corrosion. On the contrary, fibres in the interior region of specimen showed no sign of corrosion. Such result was expected because the high density of the matrix does not allow water and oxygen penetration in the specimen. Therefore, only a very superficial corrosion was observed. No corrosion occurred in specimens with no chlorides.

## 4. Conclusions

- A decrease in the first crack load and maximum load values were observed in specimens from mixes containing chlorides. The difference between the load of specimens with and without chloride remains constant over time. The most probable cause is the influence of chlorides on the cement hydration process.
- A decrease is also observed in the residual load, whose reduction becomes more evident over time. The influence of fibre corrosion may not be discarded and additional studies are necessary for a proper explanation of this process.
- The visual inspections of specimens after the 3-point bending tests confirm that corrosion

occurred only in fibres located close to the surface. Most of the cross-section showed no signs of corrosion, despite the high chloride content. This confirms that the low diffusion coefficient of the matrix limits the availability of oxygen and water required to activate the corrosion, thus protecting the fibres.

## Acknowledgements

The first author would like to thank CAPES (CAPES Foundation, Ministry of Education of Brazil, process 13117/2013-00) for the scholarship granted.

## References

- ABBAS, S. **Structural and durability performance of precast segmental tunnel linings**. 2014. Ph.D. (thesis in Civil and Environmental Engineering) - University of Western Ontario, 2014.
- ASSOCIACIÓN ESPAÑOLA DE NORMALIZACIÓN Y CERTIFICACIÓN. **UNE-EN 196-1**: Métodos de ensayo de cementos - Parte 1: Determinación de resistencias mecánicas. Madrid: AENOR, 2005.
- ALIZADE, E.; JANDAGHI ALAEE, F.; ZABIHI, S. Effect of steel fiber corrosion on mechanical properties of steel fiber reinforced concrete. **Asian Journal of Civil Engineering**, v. 17, n. 2, p. 147-158, 2016.

- ALVAREZ, A. B. **Characterization and modelling of FSRC elements**. 2013. Ph.D. (thesis in Construction Engineering) - Polytechnic University of Catalonia, 2013.
- BALOUCHE, S. U.; FORTH, J. P.; GRANJU, J.-L. Surface corrosion of steel fibre reinforced concrete. **Cement and Concrete Research**, v. 40, n. 3, p. 410-414, 2010.
- BERROCAL, C. G.; LUNDGREN, K.; LÖFGREN, I. Corrosion of steel bars embedded in fibre reinforced concrete under chloride attack: State of the art. **Cement and Concrete Research**, v. 80, p. 69-85, 2016.
- CARRILLO, J.; PULIDO, J. C.; APERADOR, W. Flexural mechanical properties of steel fiber reinforced concrete under corrosive environments. **Revista Ingeniería de Construcción**, v. 32, p. 59-72, 2017.
- DAUBERSCHMIDT, C. **Untersuchungen zu den Korrosionsmechanismen von Stahlfasern in chloridhaltigem Beton**. 2006. Ph.D. (thesis in Engineering Sciences) - Faculty of Civil Engineering of RWTH Aachen University, 2006.
- DI PRISCO, M. Preface: FRC: Structural applications and standards. **Materials and Structures/Materiaux et Constructions**, v. 42, n. 9, p. 1169-1171, 2009.
- DI PRISCO, M.; PLIZZARI, G.; VANDEWALLE, L. Fibre reinforced concrete: New design perspectives. **Materials and Structures/Materiaux et Constructions**, v. 42, n. 9, p. 1261-1281, 2009.
- DOUSTI, A. et. al. Binding of externally supplied chlorides in micro silica concrete under field exposure conditions. **Cement and Concrete Composites**, v. 33, n. 10, p. 1071-1079, 2011.
- FRAZÃO, C. et. al. Corrosion effects on pullout behavior of hooked steel fibers in self-compacting concrete. **Cement and Concrete Research**, v. 79, p. 112-122, 2016.
- GRAEFF, A. et. al. Corrosion Durability of Recycled Steel Fibre Reinforced Concrete. **Intersections/Intersectii**. v.6, n.4, p. 77-89, 2009.
- GRANJU, J. L.; BALOUCHE, S. U. Corrosion of steel fibre reinforced concrete from the cracks. **Cement and Concrete Research**, v. 35, n. 3, p. 572-577, 2005.
- KIM, B.; BOYD, A. J.; LEE, J. Y. Durability performance of fiber-reinforced concrete in severe environments. **Journal of Composite Materials**, v. 45, n. 23, p. 2379-2389, 2011.
- MANGAT, P. S.; GURUSAMY, K. Long-term properties of steel fibre reinforced marine concrete. **Materials and Structures**, v. 20, n. 4, p. 273-282, 1987.
- MANGAT, P.S.; MOLLOY, B. Size effect of reinforcement on corrosion initiation. In: 5th International RILEM Symposium on Fibre-Reinforced Cement Composites. **Proceedings**. Lyon, France: 2000
- MANTEGAZZA, G.; GATTI, A. Aspects of durability of fiber reinforced concrete: workability and stress-corrosion. In: 6th RILEM Symposium on Fiber Reinforced Concrete (FRC) - BEFIB 2004. **Proceedings**. Varenna, Italy: 2004
- NAAMAN, A. E. Evaluation of steel fibers for applications in structural concrete. In: 6th RILEM Symposium on Fiber Reinforced Concrete (FRC). **Proceedings**. BEFIB 2004, p. 389-400, 2004.
- NORDSTRÖM, E. **Durability of sprayed concrete steel fibre corrosion in cracks**. 2005. Ph.D. (thesis in Structural Engineering) - Lulea University of Technology, 2005.
- RIZZUTI, L.; BENCARDINO, F. Effects of fibre volume fraction on the compressive and flexural experimental behaviour of SFRC. **Contemporary Engineering Sciences**, v. 7, n. 8, p. 379-390, 2014.
- SADEGHI-POUYA, H. et. al. Corrosion durability of high-performance steel fibre reinforced concrete. In: Third International Conference on Sustainable

Construction Materials and Technologies.  
**Proceedings**. Kyoto, Japan: 2013.

SERNA, P.; ARANGO, S. Evolution of the flexural behaviour of precracked SFRC in marine environment. In: 17th International RILEM Symposium on Fibre Reinforced Concrete: Design and Applications.  
**Proceedings**. Chennai, India: 2008.

TAYLOR, H. F. W. **Cement chemistry**. 2<sup>nd</sup> ed. Londres: Thomas Telford Publishing, 1997.

THOMAS, J.; RAMASWAMY, A. Mechanical properties of steel fiber-reinforced concrete. **Journal of Materials in Civil Engineering**, v. 19, p. 385-392, 2007.

VIEIRA, M. M.; CAVALARO, S. H. P.; AGUADO, A. Surface corrosion in uncracked high-performance steel fibre reinforced cementitious composites. **Journal of Urban Technology and Sustainability**, v. 4, p. 4-12, 2021.